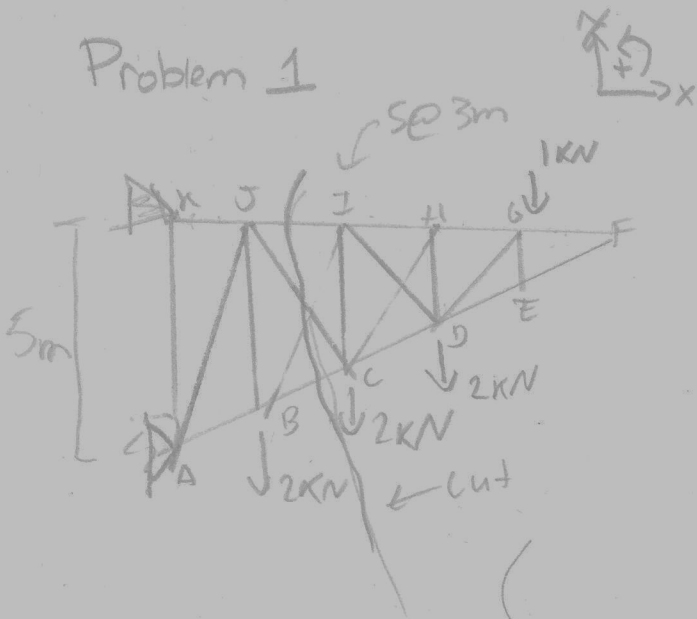


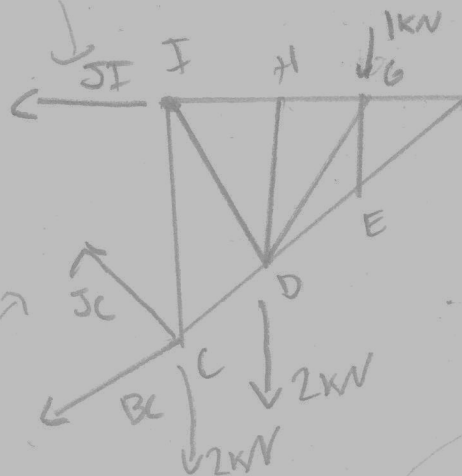
# Problem 1



Find internal forces for  
JI and JC.

I make a cut through JI and JC so I can solve in one go.  
It does not matter what side I choose because of equilibrium, but I choose the right because I do not know the reactions @ K and A.

$$5 \left( \frac{3}{5} \right) = 3$$



Three unknowns:  
JI, JC, BC

Three equations!

$$\sum M_C = 0$$

$$J_I(3) - 2(3) - 1(6) = 0$$

$$J_I = 4 \text{ kN T}$$

$$\sum F_x = 0$$

$$-J_C \cos \theta - B_C \cos \phi - J_I = 0$$

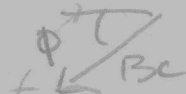
$$J_C \cos \theta + B_C (\cos \phi) + J_I = 0$$

$$B_C = \frac{1}{\cos \phi} (-J_I - J_C \cos \theta)$$

I assume in tension, if neg. then we know its in compression

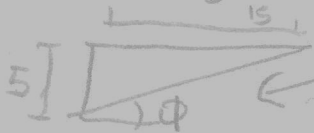


$$\theta = 45^\circ$$



I didn't calculate

$$\text{but } \tan \phi = \frac{5}{15} = \frac{1}{3}$$



$$\sum F_y = 0$$

$$J_C \sin \theta - B_C \sin \phi - 2 - 2 - 1 = 0$$

$$J_C \sin \theta + \tan \phi (J_I + J_C \cos \theta) = 5$$

$$J_C \sin \theta + \frac{1}{3} (J_I + J_C \cos \theta) = 5$$

$$J_C (\sin \theta + \frac{1}{3} \cos \theta) = 5 - \frac{1}{3} J_I$$

$$J_C = \frac{5 - \frac{1}{3} J_I}{\sin \theta + \frac{1}{3} \cos \theta} = \frac{5 - \frac{1}{3}(4)}{\frac{\sqrt{2}}{2} + \frac{\sqrt{2}}{2}(\frac{1}{3})}$$

$$J_C = 3.9 \text{ kN T}$$

$$\begin{aligned} J_I &= 4 \text{ kN T} \\ J_C &= 3.9 \text{ kN T} \end{aligned}$$

Problem 2

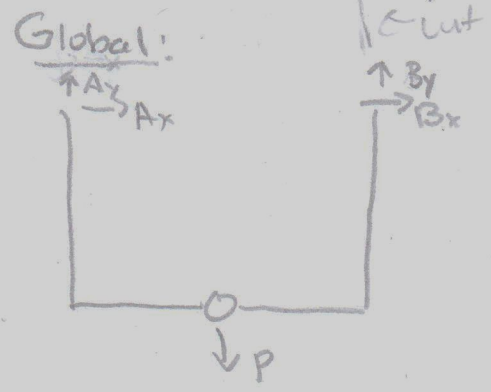
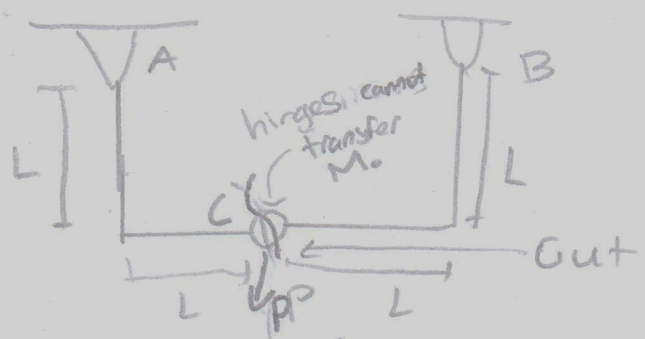
For frames, we solve by making cuts.

I make a cut at the hinge to solve the problem.

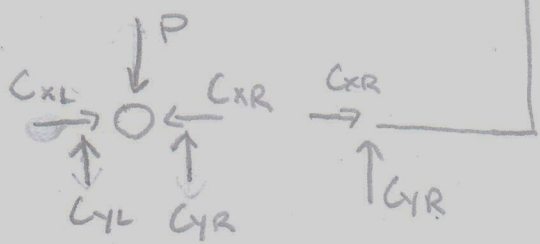
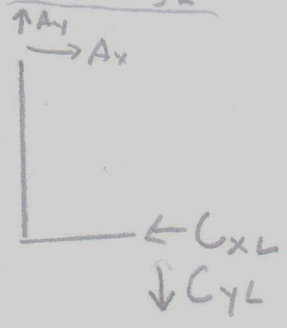
In general when I make cut I have the following:

However since there is a hinge @ C and a pin at A, I have this.

There is no moment @ C or A b/c pins and hinges don't transfer moment.



Sections:



In the global FBD I have 4 unknowns and 3 equations. However, I have a hinge so I can solve the equations.

Let's see why. When I count all of the unknowns in my section I get:  $A_y, A_x, C_{xL}, C_{yL}, C_{xR}, C_{yR}, B_y, B_x$ , or 8 unknowns.

From the sections on the left and right I get 3 equations per sections, so 6 in total. From the point C, I get

2 equations (there is no moment equation). In total I have 8 equations and 8 unknowns so I can solve, or the problem is statically determinant. If I did not have the hinge, I would have the moments  $C_{mL}$  and  $C_{mR}$  and only one more equation at the point C; the moment equation. This would mean I have 10 unknowns and 9 equations.  $\rightarrow$

In short, the addition of this hinge makes the problem statically determinate.

Let's solve:

Global:

$$\sum F_x = 0$$

$$A_x = -B_x$$

$$\sum F_y = 0$$

$$A_y + B_y = P$$

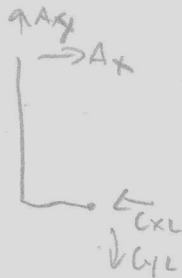
$$A_y = P/2$$

$$\sum M_A = 0$$

$$B_y(2L) - P(L)$$

$$B_y = P/2$$

Left side:



$$\sum M_C = 0$$

$$A_x(L) + A_y(L) = 0$$

$$A_x = -P/2$$

Back to global:

$$A_x = -B_x$$

$$B_x = P/2$$

$$A_x = -P/2$$

$$A_y = P/2$$

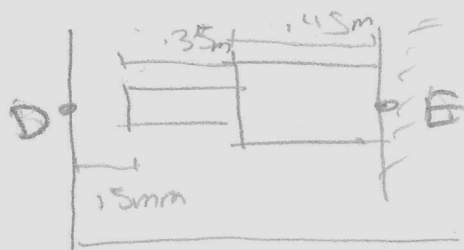
$$B_x = -P/2$$

$$A_y = P/2$$



$$P_1(L) - P_2(L)$$

### Problem 3:



Bronze:

$$A = 1500 \text{ mm}^2 = .0015 \text{ m}^2$$

$$E = 105 \text{ GPa} = 105 \times 10^9 \text{ Pa}$$

$$\alpha = 21.6 \times 10^{-6} / ^\circ\text{C}$$

Aluminum

$$A = 1800 \text{ mm}^2 = .0018 \text{ m}^2$$

$$E = 73 \text{ GPa} = 73 \times 10^9 \text{ Pa}$$

$$\alpha = 23.2 \times 10^{-6} / ^\circ\text{C}$$

a) Find flexibility constants

$$\frac{L}{EA} P = f P$$

↑ flexibility constant

Bronze:

$$f_B = \frac{L_B}{E_B A_B} = \frac{.35}{(.0015)(105 \times 10^9)}$$

$$f_B = 2.22 \times 10^{-9} \text{ m/N}$$

Aluminum:

$$f_A = \frac{L_A}{E_A A_A} = 3.42 \times 10^{-9} \text{ m/N}$$

→ To solve this problem I use

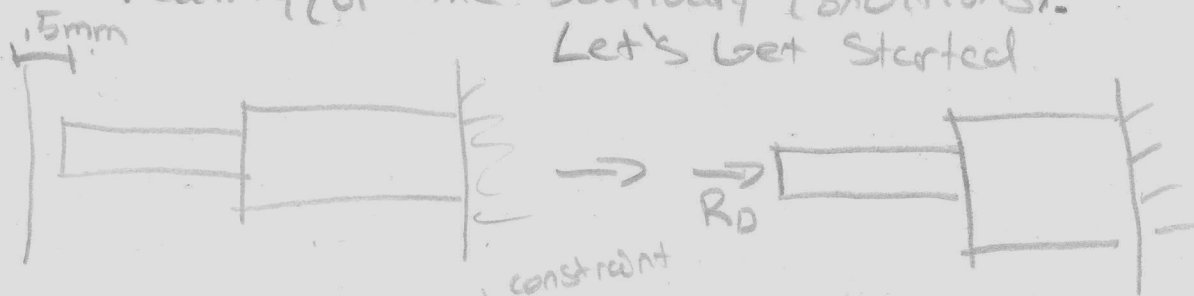
the principle of superposition. This problem is statically indeterminate if the bars stretch long enough to touch point D. We have two unknowns ( $R_D, R_E$ ) and 1 equation, ( $\sum F_x = 0$ ). However, we are able to solve this relation now because we have learned about stress and strain and how forces (statics) are related to displacements (kinematics). This relationship provides the necessary equations to solve a statically indeterminate problem.

→ For the principle of linear superposition, I take advantage of the fact that everything is linear and so I can add results together and everything is still consistent. In this principle, I isolate each cause of stress/strain, or in other words, each temperature displacement or force, and solve for its individual  $\Delta$  (displacement). I then combine them all together with a constraint equation for boundary condition. This constraint equation comes when I erase one of the boundary conditions. When I release a boundary condition, I must "replace" it with a force and displacement constraint equation, otherwise the system will not be accurately represented. In other words, the boundary condition @ point D can produce a reaction (or force) and cannot move (displacement constraint). Since, I am →

→ ... Continuation of problem 3

... Since I am removing the support @ D and nothing in this world is free, I must account for the force it could cause ( $R_D$ ) and the displacement constraint it imposes on my system will not be consistent with the original problem. Once I have done this, I break everything apart and solve for individual displacements (each caused by a force or change in temperature. I then plug that into my displacement constraint and use it to "enforce" reality (or the boundary conditions).

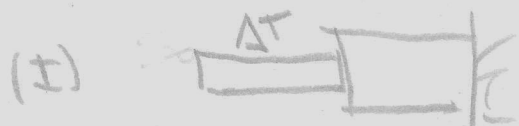
Let's get started



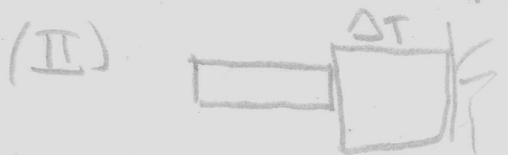
Displacement constraint

$$\Delta T = \Delta_{Temp} + \Delta_{R_D} \leq 0.5 \text{mm}$$

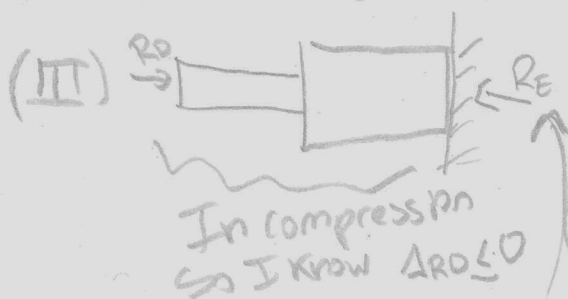
Note: if  $\Delta_{Temp} \leq 0.5 \rightarrow \Delta_{R_D} = 0$



$$\Delta_{Temp \text{ Bronze}} = \alpha_B \Delta T L_B = 7.56 \times 10^{-4} \text{m}$$



$$\Delta_{Temp \text{ Aluminum}} = \alpha_A \Delta T L_A = 10.44 \times 10^{-4} \text{m}$$



$$\Delta_{R_D} = -\Delta_{R_{\text{Bronze}}} - \Delta_{R_{\text{Alum}}} = -f_{\text{Bronze}} R_D + f_{\text{Alum}} R_D$$

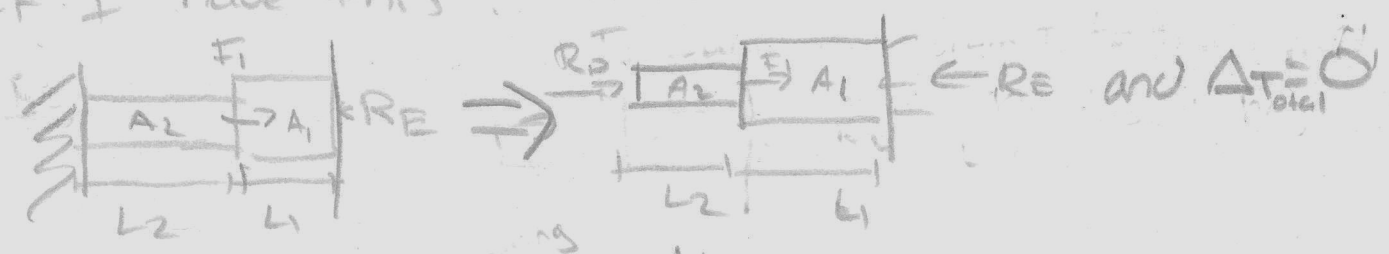
$$\Delta_{R_D} = \left( \frac{L_B}{E_B A_B} + \frac{L_A}{E_A A_A} \right) R_D$$

$R_E$  always exists.  $R_E$  is what provides the counter force so that the shape is in equilibrium. This is why when using superposition the length the force "strains" is from the point the force acts to the support.

See the next page for an example

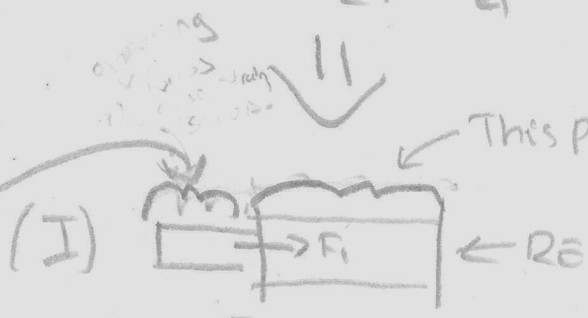
Brief Interlude to explain how Superposition accounts for strain.

If I have this!



assume the same  $E$ .

No opposing forces to cause strain.



This part is in compression so only this part receives strain from  $F_1$  while solving via the linear superposition method.

In compression  $\Delta_1 = -\frac{F_1 L_1}{A_1 E}$



This whole thing is now in compression

$\Delta_2 = -\frac{R_D L_2}{A_2 E} = -\frac{R_D L_1}{A_1 E}$   
In compression

$\Delta_1 + \Delta_2 = 0$

$-\frac{F_1 L_1}{A_1 E} = R_D \left( \frac{L_2}{A_2 E} + \frac{L_1}{A_1 E} \right)$

$R_D = -\frac{L_1}{A_1} \left( \frac{A_2 A_1}{L_2 A_1 + L_1 A_2} \right)$

BACK TO SOLVING:

$(\Delta_{temp\ bronze} + \Delta_{temp\ aluminum}) + \Delta_{R_D} = 0.0005$   
Sub in values.

$(-7.56 \times 10^{-4} + 10.44 \times 10^{-4}) - R_D (2.22 \times 10^{-9} + 3.42 \times 10^{-9}) = 0.0005$

$-R_D (5.64 \times 10^{-9}) = -0.0013$

$R_D = 230 \text{ KN}$  next page

Since  $R_D = 230 \text{ kN}$

$$\Sigma F_x = 0 \Rightarrow R_E = R_D = 230 \text{ kN}$$

b) compressive force of bar



$$R_D \rightarrow \square \leftarrow P \quad \boxed{P = 230 \text{ kN (compression)}}$$

\* Note, I did not check this value, but the hw had  $\Delta T = 950$  and this problem has  $\Delta = 1000$ . From hw  $R_D = 217 \text{ kN}$ , and for this problem  $R_D = 230 \text{ kN}$  so I am pretty sure it is right.

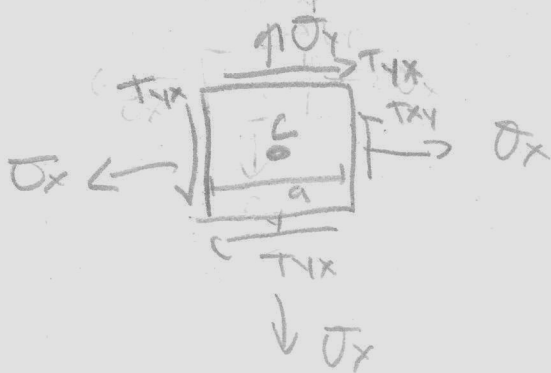
c) Stress:

$$\sigma_{\text{bronze}} = \frac{P_{\text{bronze}}}{A_{\text{bronze}}} = 153 \times 10^3 \text{ kPa (compression)}$$

$$\sigma_{\text{aluminum}} = \frac{P_{\text{Al}}}{A_{\text{Al}}} = 128 \times 10^3 \text{ kPa (compression)}$$

# Problem 4

## A. Possible Equilibrium Stress States



$$\sum M_c = 0$$

$$T_{yx}(a) - T_{xy}(a) = 0$$

$$T_{yx} = T_{xy} \checkmark$$

Stress tensor:

$$\begin{bmatrix} \sigma_x & \tau_{xy} \\ \tau_{yx} & \sigma_y \end{bmatrix}$$

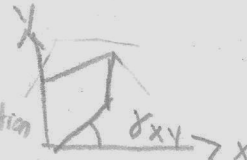
$$T_{yx} = T_{xy}$$

So the matrix must be symmetric for equilibrium.  $\sigma_y$  and  $\sigma_x$  equilibrate themselves

b and d

## B) Shear strain:

(a) relative elongation (that's normal stress **WRONG!**)

b) change of angle (Yes) → definition 

c). has nothing to do with temperature (correct)

d) has something to do with change in shape (correct)  
↳ angle changes

e) something to do w/ volume (Wrong! shape changes not volume)





$$I_{AA'} = \frac{\pi r^4}{8}$$

Parallel axis theorem only works about the centroid!

As a result we need  $I_{cx}$

$$I_x = I_{cx} + Aa^2$$

$$I_{AA'} = I_{cx} + Aa^2$$

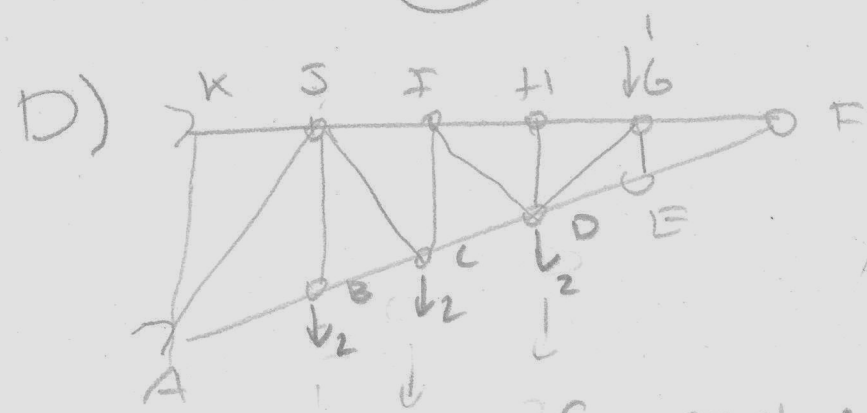
$$I_{cx} = I_{AA'} - Aa^2$$

$$I_{xx} = I_{cx} + Ab^2$$

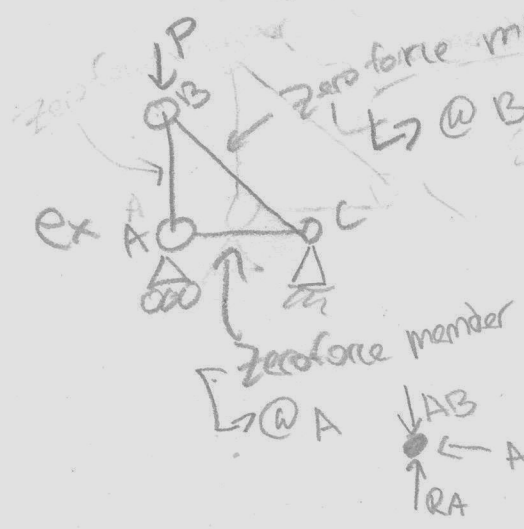
$$I_{xx} = I_{AA'} - Aa^2 + b^2(A)$$

$$I_{xx} = \frac{\pi r^4}{8} - a^2A + b^2A$$

(1)



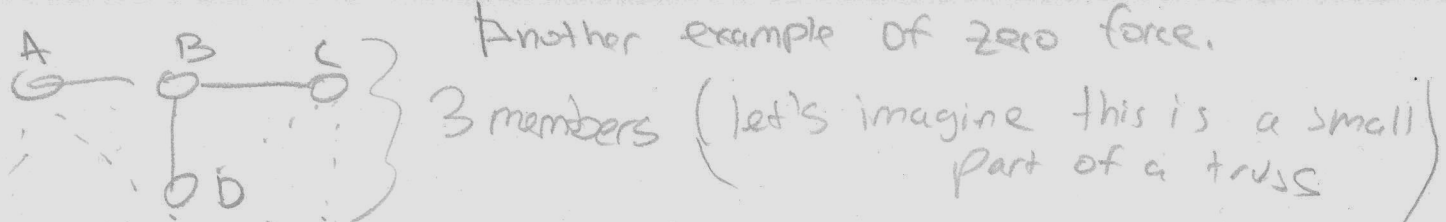
Examples of zero force members:



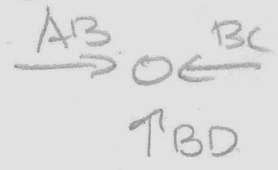
$\sum F_x = 0$   
 $Bc \cos \theta = 0$   
 $ABc = 0$

$\sum F_x = 0$   
 $Ac = 0$

Zero force member occur when two members come together and there is no reaction or external force to oppose the forces from a member or when 3 members come together and two of them are in the same plane and there is no external force or reaction to oppose one of the members forces



Another example of zero force.  
 The rest of the truss structure is not shown besides all the members that connect @ B.



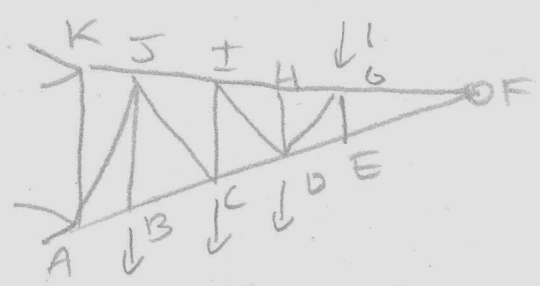
$$\sum F_x = 0$$

$$AB = BC$$

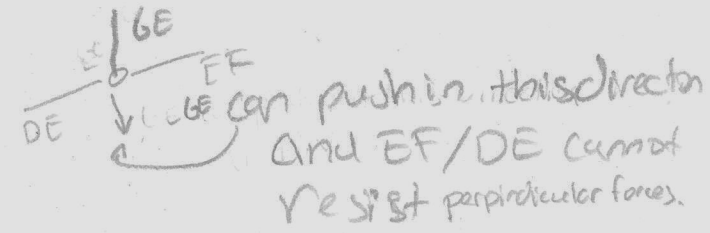
$$\sum F_y = 0$$

$$BD = 0 \text{ (Zero Force member)}$$

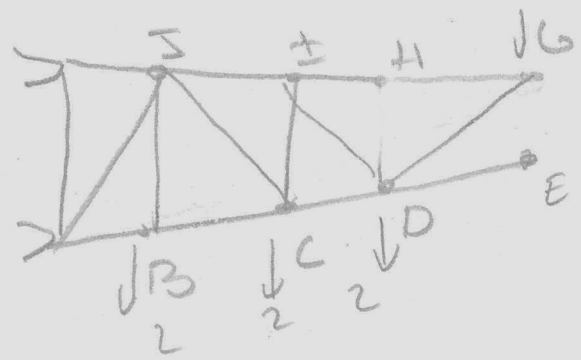
You should be able to just see this on the test! (no  $\sum F_x = 0, \sum F_y = 0$ )



From observation:  
 → GF, FE are zero force members  
 → GE is a zero force member  
 → HD is a zero force member  
 → DE is a zero force member  
 → cannot find anymore (DE does not cause any other zero force members)



Let's redraw?



→ DE is dangling → zero force  
 → cannot find anymore (DE does not cause any other zero force members)

GF, FE, GE, HD, DE are zero force

So  
a, b, d, e, f