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UNIVERSITY OF CALIFORNIA
Department of Materials Science & Engineering

Professor Ritchie

MSE 113
Mechanical Behavior of Materials

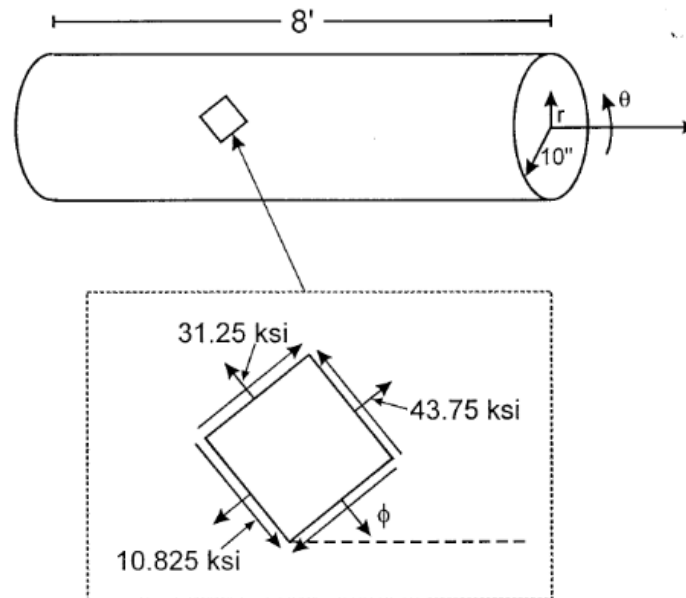
Midterm Exam #1

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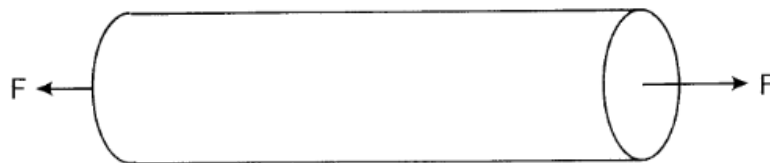
Problem	Total	Score
1	35	
2	30	
3	35	

Problem 1 35 points



Consider a thin walled, cylindrical tank made of 1080 steel under internal pressure with inside radius, $r = 10$ in, wall thickness, $t = 0.05$ in, and a length of 8 feet. Given the stresses acting on an element in the θ - z plane of the cylinder wall oriented at an angle ϕ from the coordinate axes,

- Find the angle ϕ that this element is oriented relative to the coordinate axes.
- Find $\sigma_{\theta\theta}$, σ_{zz} , and σ_{rr} .
- Find the internal pressure, P , in the cylinder.
- Find the axial force, F , that needs to be applied to the cylinder to reach the onset of yielding given that 1080 steel has a tensile yield strength of 85 ksi and ultimate tensile strength of 140 ksi. (Use Tresca criteria.) Hint, this is a 3D problem.



Solution

a.

For the following equations consider: $\sigma_x = 43.75$, $\sigma_y = 31.25$, $\tau_{xy} = 10.825$

$$\text{Center: } \frac{\sigma_x + \sigma_y}{2} = 37.5 \text{ ksi}$$

$$\text{Radius: } \sqrt{\left(\frac{\sigma_x + \sigma_y}{2}\right)^2 + \tau_{xy}^2} = 12.5 \text{ ksi}$$

$$\tan(2\psi) = \frac{2\tau_{xy}}{\sigma_x - \sigma_y} = 1.732$$

$$\rightarrow \psi = 30^\circ$$

b.

$$\sigma_{max} = \sigma_{\theta\theta} = 37.5 + 12.5 = \mathbf{50 \text{ ksi}}$$

$$\sigma_{min} = \sigma_{zz} = 37.5 - 12.5 = \mathbf{25 \text{ ksi}}$$

$$\sigma_{rr} \approx \mathbf{0}$$

c.

$$\sigma_{\theta\theta} = \frac{pr}{t}$$

$$p = \frac{\sigma_{\theta\theta} t}{r} = \frac{50,000 \text{ psi} * .05''}{10''} = \mathbf{250 \text{ psi}}$$

or

$$\sigma_{zz} = \frac{pr}{2t}$$

$$p = \frac{2\sigma_{\theta\theta} t}{r} = \frac{2 * 25,000 \text{ psi} * .05''}{10''} = 250 \text{ psi}$$

d.

Use superposition: Tension + Pressure

$$\tau_{max} = k = \frac{85 + 0}{2} = 42.5 \text{ ksi}$$

$$\rightarrow \sigma_{tensile \text{ yield}} = 2 * \tau_{max} = 85 \text{ ksi}$$

$$\text{For yielding: } \sigma_{zz} = 85 \text{ ksi} = \sigma_{zz}^{tension} + \sigma_{zz}^{pressure}$$

$$\rightarrow \sigma_{zz}^{tension} = 85 - 25 = 60 \text{ ksi}$$

$$\sigma_{zz}^{tension} = \frac{F}{Area} = \frac{F}{2\pi r t}$$

$$\rightarrow \mathbf{F} = \sigma_{zz}^{tension} * 2\pi r t = 60 \text{ ksi} * 2 * \pi * 10'' * .05'' = \mathbf{188.5 \text{ kips}}$$

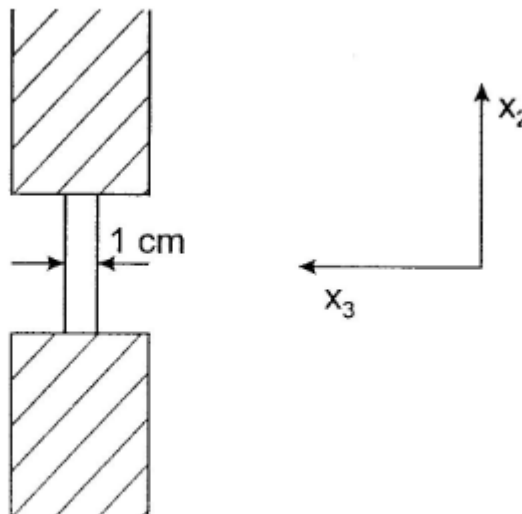
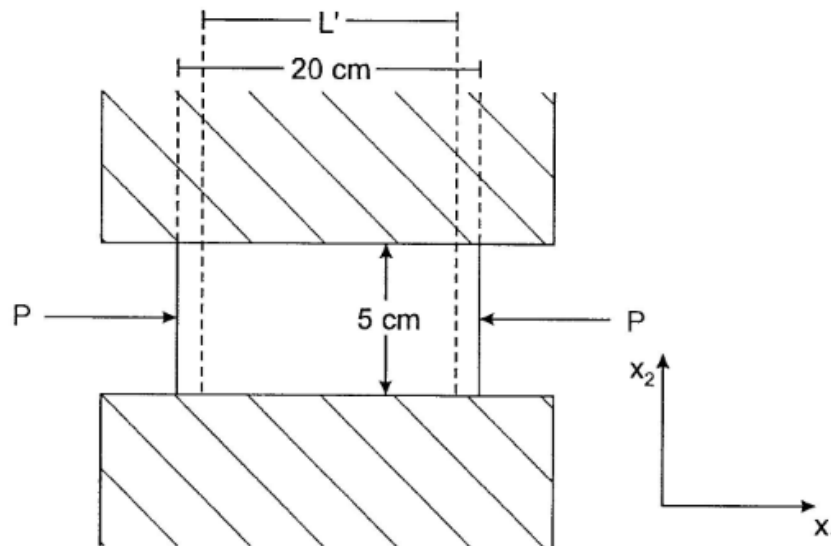
Problem 2 30 points

Suppose you have a long thin plate made of 6061-T4 aluminum between two rigid (non-deforming) walls acted on by a compressive force P of 10,000 N in the x_1 direction. The plate has original dimensions: $L = 20$ cm, $b = 5$ cm, and $t = 1$ cm.

Find σ_{22} , ϵ_{33} , and the new length L' .

Properties of 6061-T4 Aluminum

Young's Modulus (GPa)	Shear Modulus (GPa)	Poisson's Ratio	Tensile Yield Strength (MPa)	Ultimate Tensile Strength (MPa)
70	26.3	1/3	145	240



Solution

$$\sigma_{11} = \frac{-P}{bt} \quad \text{negative because it is a compressive force}$$

$$\sigma_{11} = \frac{-10,000 \text{ N}}{0.05 \text{ m} * 0.01 \text{ m}}$$

This will produce a stress in the 2 direction, σ_{22} , because of the constraint, $\epsilon_{22} = 0$. There won't be a stress in the 3 direction, $\sigma_{33} = 0$, because there is no constraint. Instead the material will deform in this direction, ϵ_{33} .

$$\text{Note } \sigma_{33} = \sigma_{12} = \sigma_{13} = \sigma_{23} = 0$$

Setup strain equations:

$$\epsilon_{11} = \frac{1}{E}(\sigma_{11} - \nu\sigma_{22}) \quad \epsilon_{22} = \frac{1}{E}(\sigma_{22} - \nu\sigma_{11}) \quad \epsilon_{33} = \frac{1}{E}(-\nu(\sigma_{11} + \sigma_{22}))$$

$$\epsilon_{12} = \epsilon_{13} = \epsilon_{23} = 0$$

$$\text{From } \epsilon_{22} = 0 \rightarrow \sigma_{22} = \nu\sigma_{11} = \frac{1}{3} * -20 \text{ MPa} = -6.67 \text{ MPa}$$

Then,

$$\epsilon_{33} = \frac{1}{E}(-\nu(\sigma_{11} + \sigma_{22}))$$

$$\epsilon_{33} = \frac{1}{70,000 \text{ MPa}} \left(-\frac{1}{3}(-20 \text{ MPa} - 6.67 \text{ MPa}) \right)$$

$$\epsilon_{33} = 1.27 * 10^{-4}$$

Finally,

$$\epsilon_{11} = \frac{1}{E}(\sigma_{11} - \nu\sigma_{22})$$

$$\epsilon_{11} = \frac{1}{70,000 \text{ MPa}} \left(-20 \text{ MPa} + \frac{6.67 \text{ MPa}}{3} \right) = -2.54 * 10^{-4}$$

$$\text{Where, } \epsilon_{11} = \frac{\Delta L}{L}$$

$$\rightarrow \Delta L = L * \epsilon_{11} = 20 \text{ cm} * -2.54 * 10^{-4} = -0.005 \text{ cm}$$

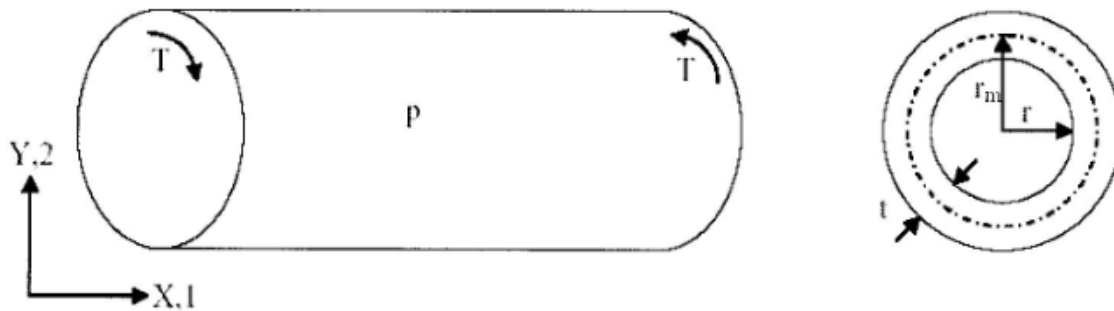
$$L' = L + \Delta L = 20 \text{ cm} - 0.005 \text{ cm} = 19.995 \text{ cm}$$

Problem 3 35 points

The thin-walled cylinder, shown below, has an internal pressure of $p = 1000$ kPa, and is subjected to a twist of $T = 10$ MNm. The inner radius of the cylinder is 2 m with a wall thickness of 20 mm. The shear stress on a thin-walled tube can be approximated by:

$$\tau = \sigma_{\theta z} = \frac{T}{2 \pi r_m^2 t}$$

where T is the applied torsional moment, r_m is the radius to the median line, and t is the thickness of the cylinder. See below:



- a) Assuming that the ends have no effect on the stresses near the center of the cylinder,
 - i. Determine the principal stresses and show them on a sketch of a properly oriented element, i.e. rotated by the principal angle.
 - ii. Determine the maximum shear stress.
- b) Is there any evidence of plastic yielding in the cylinder if the yield strength of the material used is 180 MPa? (Consider only the θ - z plane, i.e. 2D)
- c) If the cylinder is punctured to leave a tiny pinhole in the wall thickness, check if yielding will occur at the edge of the hole using the Tresca and von Mises criteria.

Solution

$$r_m = r + \frac{t}{2} = 2 + \frac{0.02}{2} \text{ m} = 2.01 \text{ m}$$

$$\tau = \sigma_{\theta z} = \frac{T}{2 \pi r_m^2 t} = \frac{10 \cdot 10^6 \cdot \text{N} \cdot \text{m}}{2 \pi (2.01 \text{ m})^2 \cdot 0.02 \text{ m}} = 1.97 \cdot 10^7 \text{ Pa} = 19.7 \text{ MPa}$$

$$\sigma_{\theta\theta} = \frac{pr}{t} = \frac{1 \text{ MPa} \cdot 2 \text{ m}}{0.02 \text{ m}} = 100 \text{ MPa}$$

$$\sigma_{zz} = \frac{pr}{2t} = \frac{1 \text{ MPa} \cdot 2 \text{ m}}{2 \cdot 0.02 \text{ m}} = 50 \text{ MPa}$$

a.

For the following equations,

$$\sigma_x = \sigma_{\theta\theta} = 100 \text{ MPa}$$

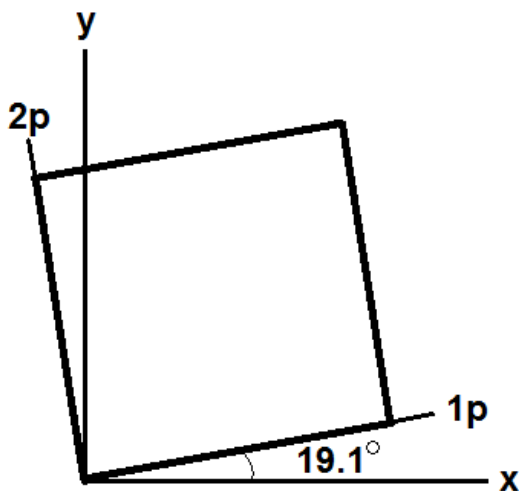
$$\sigma_y = \sigma_{zz} = 50 \text{ MPa}$$

$$\tau_{xy} = \tau = 19.7 \text{ MPa}$$

$$\sigma_{1p} = \frac{\sigma_x + \sigma_y}{2} + \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} = 106.8 \text{ MPa}$$

$$\sigma_{2p} = \frac{\sigma_x + \sigma_y}{2} - \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} = 43.2 \text{ MPa}$$

$$\tan(2\theta_p) = \frac{2\tau_{xy}}{\sigma_x - \sigma_y} \rightarrow \theta_p = 19.1^\circ$$



$$\tau_{max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2} = 31.8 \text{ MPa}$$

b.

Tresca

$$k = \frac{\sigma_y}{2} = \frac{180 \text{ MPa}}{2} = 90 \text{ MPa} > \tau_{max} = 31.8 \text{ MPa}$$

Therefore, does **not** yield.

Von Mises

Consider the stress state in the θ -z plane, called the 1-2 plane here. Note that the stresses in the 3rd direction are zero since this is a 2D problem.

$$\sigma = \sqrt{\frac{1}{2}[(\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{33} - \sigma_{11})^2] + 3(\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{31}^2)}$$

$$\sigma = \sqrt{\frac{1}{2}[(100 - 50)^2 + (50 - 0)^2 + (0 - 100)^2] + 3(19.7^2 + 0^2 + 0^2)}$$

$$\sigma = 93.1 \text{ MPa} < \sigma_y = 180 \text{ MPa}$$

Therefore, does **not** yield.

c.

$$S_1 = \sigma_{1p} = 106.8 \text{ MPa} \quad S_2 = \sigma_{2p} = 43.2 \text{ MPa}$$

$$\sigma_{A\theta\theta} = 3S_2 - S_1 = 22.7 \text{ MPa} \quad \sigma_{B\theta\theta} = 3S_1 - S_2 = 277.3 \text{ MPa}$$

$\sigma_{Arr} = \sigma_{Brr} = \sigma_{Ar\theta} = \sigma_{Br\theta} = 0$ Note that at the edge of a hole there is no material radially inward. If there is no material present, then there can't be a stress inward. In order to have force balance, there can't be any radial stresses at the edge of a hole.

At point A

Tresca

$$\tau_{max} = \frac{\sigma_{A\theta\theta} - \sigma_{Arr}}{2} = 11.34 \text{ MPa} < 90 \text{ MPa} = k$$

Von Mises

$$\sigma = \sqrt{\frac{1}{2}[(22.7 - 0)^2 + (0 - 0)^2 + (0 - 22.7)^2] + 3(0^2 + 0^2 + 0^2)} = 11.34 < k$$

Yielding does not occur at point A.

At point B

Tresca

$$\tau_{max} = \frac{\sigma_{A\theta\theta} - \sigma_{Arr}}{2} = 138.7 \text{ MPa} > 90 \text{ MPa} = k$$

$$\sigma = \sqrt{\frac{1}{2}[(138.7 - 0)^2 + (0 - 0)^2 + (0 - 138.7)^2] + 3(0^2 + 0^2 + 0^2)} = 138.7 \text{ MPa} > k$$

Yielding occurs at Point B.

