

CE 120 Structural Engineering Examination #1

Solution 1

a) Cutting through C and between D and G: unknowns = $U=6$ (3 reactions, 2 internal forces at C and force in DG); Equations = $E=2 \times 3 = 6$ for 2 FBD's

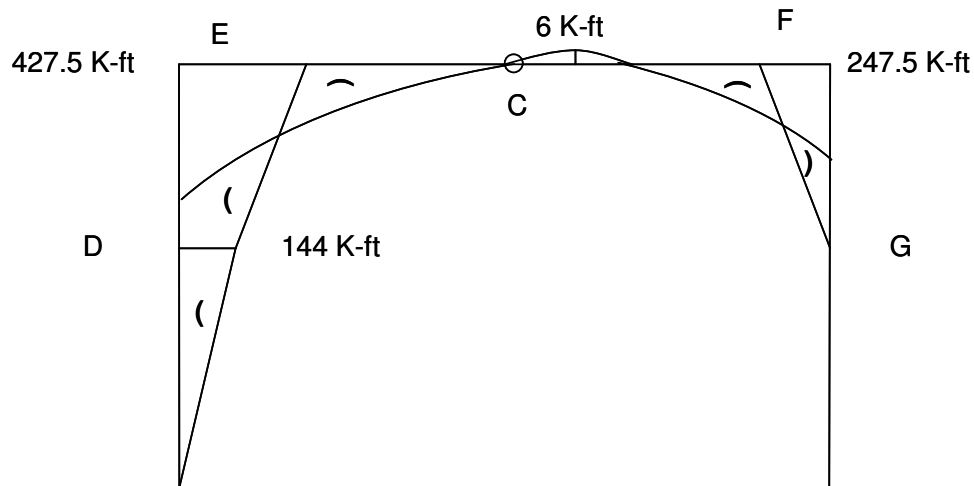
$n=U-E=0$; Statically determinate and stable (reactions are not parallel or intersecting).

If member DG is absent, it is unstable as $n=-1$.

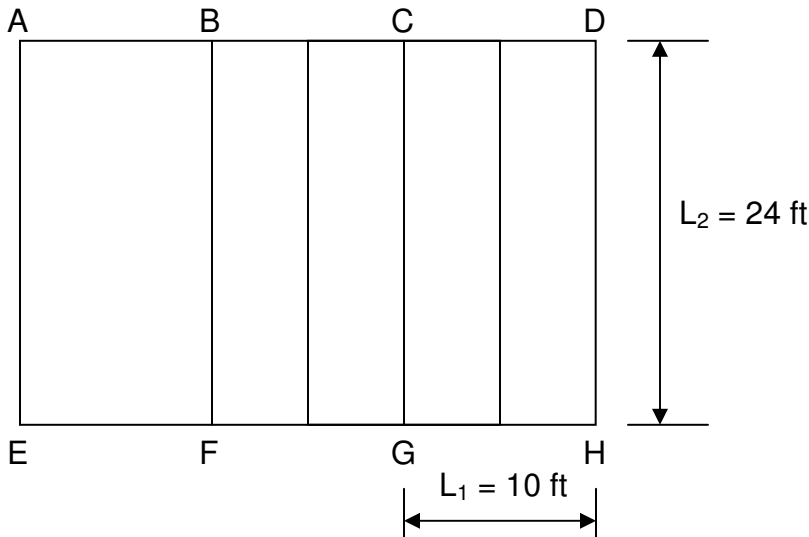
b)

$\sum M_A = 0$ $\Rightarrow B_y = 39 \text{ K}$ $\sum F_y = 0$ $\Rightarrow A_y = 51 \text{ K}$ $\sum F_x = 0$ $\Rightarrow A_x = 12 \text{ K}$	$\sum M_C = 0$ $\Rightarrow 9 F_{DG} + (12)(21) + (45)(7.5) - (51)(15) = 0$ $\Rightarrow F_{DG} = 19.5 \text{ K}$ $\sum F_y = 0$ $\Rightarrow C_y = -6 \text{ K}$ $\sum F_x = 0$ $\Rightarrow C_x = 31.5 \text{ K}$

Bending Moment Diagram:



Solution 2



$$\gamma_{\text{concrete}} = 150 \text{ lb/ft}^3$$

$$\gamma_{\text{steel}} = 490 \text{ lb/ft}^3$$

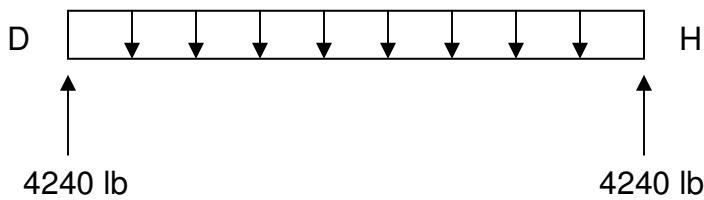
a) Beam DH:

Dead load from the slab = $(150)(5/12)(5) = 312.5 \text{ lb/ft}$

Area of cross section of the beam = $[(2)(4)(1)+(8)(0.5)]/144 = 1/12 \text{ ft}^2$

Self weight of the beam = $490/12 = 40.83 \text{ lb/ft}$

$$312.5 + 40.83 = 353.33 \text{ lb/ft}$$



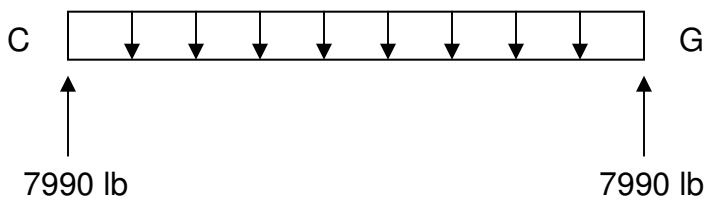
Beam CG:

Dead load from the slab = $(150)(5/12)(10) = 625 \text{ lb/ft}$

Area of cross section of the beam = $[(2)(4)(1)+(8)(0.5)]/144 = 1/12 \text{ ft}^2$

Self weight of the beam = $490/12 = 40.83 \text{ lb/ft}$

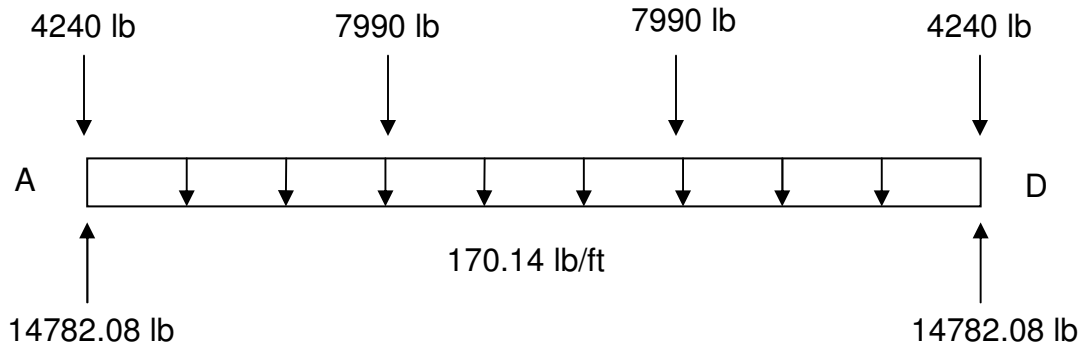
$$625 + 40.83 = 665.83 \text{ lb/ft}$$



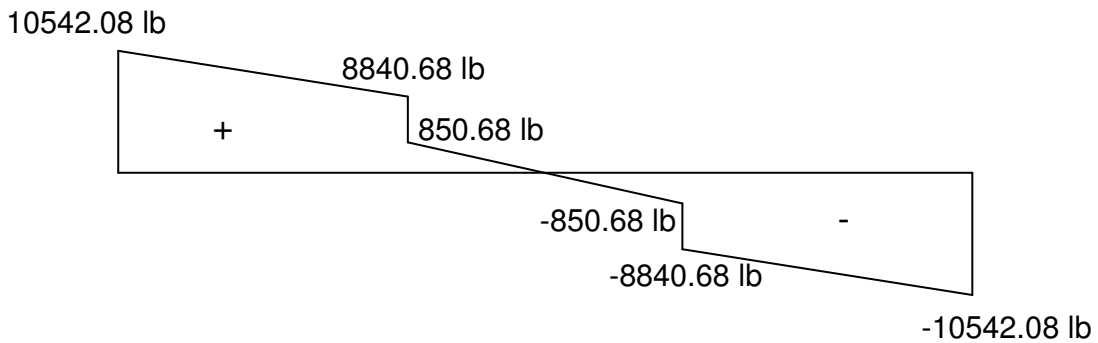
Girder ABCD:

Area of cross section of the beam = $[(2)(8)(2)+(12)(1.5)]/144 = 0.347 \text{ ft}^2$

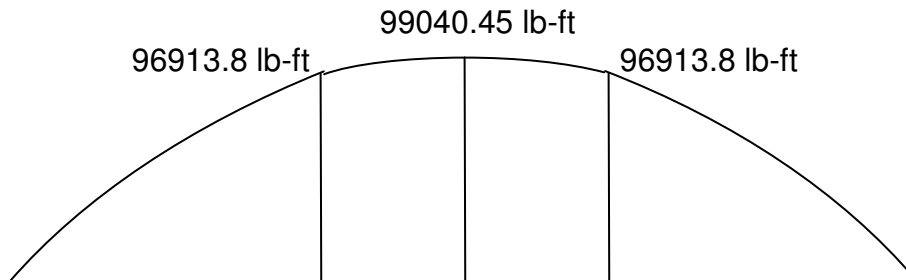
Self weight of the beam = 170.14 lb/ft



Shear Force Diagram:



Moment Diagram:



$$I = 2[(1/12)(8)(2^3) + (16)(7^2)] + (1/12)(1.5)(12^3) = 1794.67 \text{ in}^4$$

Maximum bending moment is in the center of the girder ABCD. It is 99.04 K-ft.

$$\sigma = Mc/I = (99.04)(12)(8) / (1794.67) = 5.3 \text{ ksi}$$

Maximum shear is at the ends = 10.54 K, $v = VQ / Ib$

$$\text{Shear stress at the joint (inside the flange): } v = (10.54)(16)(7)/(1794.67)(8) = 82.1 \text{ psi}$$

$$\text{Shear stress at the joint (inside the web): } v = (10.54)(16)(7)/(1794.67)(1.5) = 438.5 \text{ psi}$$

$$\text{Shear stress in center of web: } v = (10.54)[(16)(7)+(6)(1.5)(3)]/(1794.67)(1.5) = 544.2 \text{ psi}$$

